Exhibit F

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HARO, KASUNICH AND ASSOCIATES, INC.

CONSULTING GEOTECHNICAL & COASTAL ENGINEERS

Project No. M11450
17 January 2018

ANIL SETHI
5040 Peninsula Point Drive
Seaside, CA 93955
831.402.8294
anilksethi@yahoo.com

Subject: Geotechnical Investigation Update

Reference: Proposed Residential Development
401 Via del Milagro
Monterey County, California

Dear Mr. Sethi:

At your request, we are updating our 9 September 2005 (Project Number M8934) Geotechnical Investigation Report with additional recommendations and current California Building Code (CBC 2016) standards. We have reviewed our prior project file and geotechnical recommendations for this project.

Site Evaluation

Haro, Kasunich & Associates (HKA) conducted a site walk through on 16 January 2018. During the walk through, no significant surficial signs of instability were observed to warrant further investigation, despite the heavy rain of winter 2016-2017. This site is still undeveloped with grasses, shrubs, and several oak trees. No grading or improvements appear to have been undertaken since our original report. Therefore, our conclusions and recommendations from our September 2005 geotechnical investigation report for this project remain valid and are compatible with the current project scope. Additional recommendations presented in this letter, along with our September 2005 report, should be used as guidelines for preparing project plans and specifications.

California Building Code (2016) Seismic Design Parameters

The improvements should be designed in conformance with the most current California Building Code (2016 CBC). For seismic design, the soil properties at the site are classified as **Site Class "D"** based on definitions presented in section 1613.3.2 in the 2016 CBC. The longitude and latitude were determined using a satellite image generated by Google Earth Pro. These coordinates were taken from the approximate middle of the area of the proposed improvements:

Longitude = -121.77918, Latitude = 36.5798

The coordinates listed above were used as inputs in the Java Ground Motion Parameter Calculator created by the USGS to determine the ground motion associated with the maximum considered earthquake (MCE) SM and the reduced ground motion for design SD. The results are as follows:

Site Class D

SM_S= 1.443 g
SM_1= 0.780 g
SD_S= 0.962 g
SD_1= 0.520 g

A maximum considered earthquake geometric mean (MCE_G) peak ground acceleration (PGA) was estimated using the Figure 22-7 of the ASCE Standard 7-10. The mapped PGA was 0.549 g and the site coefficient F_{PGA} for Site Class D is 1.0. The MCE_G peak ground acceleration adjusted for Site Class effects is PGA_M = FPGA * PGA

$$PGA_M = 1.0 * 0.549\ g = 0.549\ g$$

Based on these considerations, the risk of substantial structural damage from earthquakes appears relatively low for well-built structures which incorporate lateral shear bracing and current California Building Code (CBC) requirements into their design and construction. These considerations will be the primary factors in reducing the potential for earthquake damage to the project in the future.

Geological Hazards

Liquefaction

During an earthquake, seismic waves travel through the earth and vibrate the ground. In cohesionless, granular material having low relative density (loose to medium dense sands for example), this vibration can disturb the particle framework leading to increased compaction of the material and reduction of pore space between the framework grains. If the sediment is saturated, water occupying the pore spaces resists this compaction and exerts pore pressure that reduces the contact stress between the sediment grains. With continued shaking, transfer of intergranular stress to pore water can generate pore pressures great enough to cause the sediment to lose its strength and change from a solid state to a liquefied state. This mechanical transformation termed liquefaction can cause various kinds of ground failure at or near the ground surface.

The liquefaction process typically occurs at depths less than 50 feet below the ground surface. Liquefaction can occur at deeper intervals, given the right conditions, however ground manifestations have been found to be relatively minor.

Based on the absence of groundwater in our test borings and our experience in the area, there is a low potential for liquefaction to occur at the project site.

Building Codes and Site Class

Project design and construction should conform to the following current building codes:

-2016 California Building Code (CBC); and
-2016 Green Building Standards Code (CAL Green)

In accordance with section 1613.3.2 of the 2016 CBC, the project site should be assigned the Site Class D.

Additional Geotechnical Recommendations

Based on review of our prior Geotechnical Investigation and associated addendums, we present the following additional recommendations to be used as guidelines for preparing project plans and specifications. All recommendations from Haro, Kasunich & Associates' (HKA) 9 September 2005 Geotechnical Investigation should be followed as well.

Conventional Spread Foundations

1. Conventional Spread Foundation criteria presented in this letter supersedes the "Conventional Shallow Foundations" section of our 9 September 2005 report.
2. Provided the building pads are prepared in accordance with recommendations in this letter and our 9 September 2005 report, the proposed residences should be supported by conventional spread foundations embedded into firm, moist, native soil.
3. The structure may be constructed by a series of benches cut into the hillside with conventional spread footings embedded into the native soil. If the footing excavations, especially along the outboard side of the cut benches, do not penetrate the loose/soft near surface soil, as determined by the soils engineer, additional preparation consisting of compaction along the bottom of the excavation or embedding the footings into a mat of engineered fill should be completed. The mat of engineered fill and compaction should be prepared in accordance with the section of the 2005 report titled "Site Grading". The mat of engineered fill should extend a minimum 18 inches below the bottom of footings. These measures would be to reduce the potential for settlement.
4. Actual footing depths and widths should be determined in accordance with anticipated use and applicable design standards, but should be a minimum of 24 inches wide and 24 inches deep. Conventional footings should be reinforced as required by the structural designer based on the actual loads transmitted to the foundation.
5. Foundations designed in accordance with the above may be designed for an allowable soil bearing pressure of 2,000 psf for dead plus live loads. This value may be increased by one-third to include short-term seismic and wind loads.
6. Lateral load resistance for structures supported on spread footings may be developed in friction between the foundation bottom and the supporting subgrade. A friction coefficient of 0.35 is considered applicable. Passive

resistance of 250 pcf may be used below a depth of 12 inches against engineered fill or in-situ silty sand.

7. Footings located adjacent to other footings or utility trenches should have their bearing surfaces founded below an imaginary 2:1 plane projected upward from the bottom edge of the adjacent footings or utility trenches.
8. Total and differential settlements under the proposed light building loads are anticipated to be less than 1 inch and ½ inch respectively.
9. All footing excavations should be thoroughly cleaned and observed by HKA prior to placing forms and steel. Observation of foundation excavations allows anticipated soil conditions to be correlated to those inferred from our investigation and to verify that the footings are in accordance with our recommendations

Site Drainage

10. Discharge of any surface or subsurface water should be located a minimum of 15 feet downslope from any keyway or foundation element, whichever is further.
11. Refer to HKA's 9 September 2005 Geotechnical Investigation for additional site drainage recommendations.

Site Grading

12. The base of keyways for permanent engineered fill slopes should be located a minimum of 5 feet from daylight measured horizontally.
13. Refer to HKA's 9 September 2005 Geotechnical Investigation for additional site grading recommendations.

Plan Review, Construction Observation and Testing

The above recommendations and attached 2002 soil report should be used as guidelines for preparing project plans and specifications. **Haro, Kasunich & Associates** should be commissioned to review project grading and foundation plans before construction and to observe, test and advise during earthwork and foundation construction. This additional opportunity to examine the site will allow us to compare subsurface conditions exposed during construction with those inferred from this investigation. Unusual or unforeseen soil conditions may require supplemental evaluation by the geotechnical engineer.

Should you have any questions concerning this letter report, please call our office.

Respectfully Submitted,

Reviewed by:

HARO, KASUNICH & ASSOCIATES, INC.

REGISTERED PROFESSIONAL ENGINEER
MOSES E. CUPRILL
No. 78904
Exp. 3/18
CIVIL
STATE OF CALIFORNIA

Moses Cuprill, P.E.
C.E. 78904

Brian R. Shedden, P.E.
C.E. 84817

BRS/MC/sr
Attachments
Copies: 4 to Addressee + email anilksethi@yahoo.com

LIMITATIONS AND UNIFORMITY OF CONDITIONS

1. The recommendations of this report are based upon the assumption the soil conditions do not deviate from those disclosed in the borings. If any variations or undesirable conditions are encountered during construction, or if the proposed construction will differ from what is planned at the time, our firm should be notified so supplemental recommendations can be given.

2. This report is issued with the understanding it is the responsibility of the owner, or his representative, to ensure the information and recommendations contained herein are called to the attention of the Architects and Engineers for the project and incorporated into the plans, and the necessary steps are taken to ensure the Contractors and Subcontractors carry out such recommendations in the field. The conclusions and recommendations contained herein are professional opinions derived in accordance with current standards of professional practice. No other warranty expressed or implied is made.

3. The findings of this report are valid as of the present date. However, changes in the conditions of a property can occur with the passage of time, whether they be due to natural processes or to the works of man, on this or adjacent properties. In addition, changes in applicable or appropriate standards occur whether they result from legislation or the broadening of knowledge. Accordingly, the findings of this report may be invalidated, wholly or partially, by changes outside our control. Therefore, this report should not be relied upon after a period of three years without being reviewed by a geotechnical engineer.

APPENDIX A

September 2005 Geotechnical Investigation Report (M8934)

GEOTECHNICAL INVESTIGATION
for
PROPOSED RESIDENTIAL DEVELOPMENT
401 Via Del Milagro
Monterey County, California

Prepared For
Gozzi Development

Prepared By
HARO, KASUNICH AND ASSOCIATES, INC.
Geotechnical & Coastal Engineers
Project No. M8934
September 2005

Project No. M8934
9 September 2005

GOZZI DEVELOPMENT
P.O. Box 223808
Carmel, California 93922

Attention: Anita Gozzi

Subject: Geotechnical Investigation

Reference: Residential Development
Lot 98 Pasadera- 401 Via Del Milagro
Monterey County, California

Dear Mrs. Gozzi:

The following report presents the results and conclusions of our Geotechnical Investigation for the proposed residential construction. This report includes design criteria and recommendations addressing the geotechnical aspects of the proposed development.

The results of our investigation indicate there are no significant geotechnical concerns at the site provided the recommendations presented in this report are followed in development of project plans and specifications.

If you have any questions concerning the data or conclusions presented in this report, please call our office.

Very truly yours,

HARO, KASUNICH & ASSOCIATES, INC.

Greg Bloom
C.E. 58819

GB/dk

Copies: 5 to Addressee
1 to Monterey Bay Engineers

Project No. M8934
9 September 2005

TABLE OF CONTENTS

Project No. M8934
9 September 2005

GEOTECHNICAL INVESTIGATION

Introduction

This report presents the results of our Geotechnical Investigation for the proposed new residential development to be located at 401 Via Del Milagro in Monterey County, California.

Purpose and Scope

The purpose of our investigation was to explore and evaluate the surface and subsurface conditions at the building site and provide geotechnical criteria for design and construction of the proposed residential development. The specific scope of our services was as follows:

1. Review the data in our files pertinent to the site.
2. Explore the subsurface conditions at the site with two (2) continuous flight-augered exploratory borings drilled to depths ranging from 10 ½ to 21 ½ feet deep.
3. Test selected soil samples to determine their pertinent engineering properties.
4. Evaluate the field and laboratory data to develop geotechnical design criteria and recommendations for site grading, building foundations, slabs-on-grade, retaining walls, and general site drainage.
5. Present the results of our investigation in a report.

Site Location and Description

The parcel located at 401 Via Del Milagro is located within the Pasadera Subdivision east of Monterey in unincorporated Monterey County, California.

The parcel is rectangular in shape and approximately 1 acre (±) in size. The site slopes to the south away from Via Del Milagro at an average gradient of approximately 20 to 30 percent.

The parcel is unimproved and vegetated with grasses, shrubs, and several oak trees.

Field Exploration

Subsurface conditions were investigated on 23 June 2005. The approximate location of the test borings are indicated on the Boring Site Plan. The borings were advanced using 4-inch diameter solid stem auger equipment mounted on a tractor mounted drill rig.

Representative soil samples were obtained from the exploratory borings at selected depths, or at major strata changes. These samples were recovered using the 3.0 inch O.D. Modified California Sampler (L) or the Standard Terzaghi Sampler (T).

The penetration resistance blow counts noted on the boring logs were obtained as the sampler was dynamically driven into the in situ soil. The process was performed by

dropping a 140-pound hammer a 30-inch free fall distance and driving the sampler 6 to 18 inches and recording the number of blows for each 6-inch penetration interval. The blows recorded on the boring logs represent the accumulated number of blows that were required to drive the last 12 inches.

The soils encountered in the borings were continuously logged in the field and described in accordance with the Unified Soil Classification System (ASTM D2486). The Logs of the Borings are included in the Appendix of this report. The Boring Logs denote subsurface conditions at the locations and time observed, and it is not warranted that they are representative of subsurface conditions at other locations or times.

Laboratory Testing

The laboratory testing program was directed toward determining pertinent engineering and index soil properties.

One sieve analysis was performed to assist in classifying the soil.

The strength parameters of the underlying earth materials were determined from field test values derived from standard penetration blow count measurements of the in situ soil.

The results of the field and laboratory testing appear on the "Logs of Test Boring" opposite the sample tested or in their respective graphs attached as part of the appendix of this report.

Subsurface Conditions

The native earth materials on the site consist of loose poorly graded sand with silt. The sand is loose in the near surface (upper 7 to 10 feet) and becomes medium dense with depth.

Groundwater

Groundwater was not encountered within the depths explored. It is anticipated that groundwater conditions may vary based on seasonal and other factors not readily apparent.

Seismicity

The following is a general discussion of seismicity in the project area. Detailed studies of geologic hazards are beyond the scope of this study.

The proposed site lies about 39 kilometers southwest of the San Andreas Fault zone. This is a major fault zone of active displacement which extends from the Gulf of California to the vicinity of Point Arena, where the fault leaves the California coastline. Between these

points, the fault is about 700 miles long. The fault zone is a break or series of breaks along the earth's crust, where shearing movement has taken place. This fault movement is primarily horizontal.

Historically, the San Andreas Fault has been the site of large earthquakes, and consequently large earthquakes can be expected in the future. The largest of the historic quakes in Northern California occurred on 18 April 1906 (mag. 8.3+). The recent 17 October 1989 earthquake was also associated with the San Andreas Fault system. This event was the second largest earthquake in Northern California this past century.

Other significant faults within the nearby vicinity include the Monterey Bay-Tularcitos Fault (4 kilometers to the northeast).

Geologic hazards review are beyond the scope of our services. The above information is general in nature and is provided only to illustrate that the property lies within a complex geologic area.

DISCUSSIONS AND CONCLUSIONS

Based on the results of our investigation, the proposed development, from a geotechnical standpoint, is feasible. The recommendations presented in this report are to be incorporated into the design and construction of the proposed development.

The proposed structures may be founded on conventional shallow foundations bearing on a minimum of 18 inches of engineered fill. Foundations should be stepped up the hillside and the base of the footings should be embedded a minimum of 10 feet from daylight measured horizontally.

RECOMMENDATIONS

The following recommendations should be used as guidelines for preparing project plans and specifications:

Site Grading

1. We request the opportunity to review project grading and foundation plans during the design phase of the project. We can then provide our opinion regarding geotechnical considerations.

2. Observation and testing services for earthwork performed at the project site should be provided by Haro, Kasunich and Associates. The observation and testing of earthwork allows for contractors compliance evaluation to project plans and specifications and our geotechnical recommendations. It also allows us the opportunity to confirm that actual soil conditions encountered during construction are essentially the same as those anticipated based on the subsurface exploration.

3. The geotechnical engineer should be notified **at least four (4) working days** prior to any site clearing or grading so that the work in the field can be coordinated with the grading contractor and arrangements for testing and observation can be made. The recommendations of this report are based on the assumption that the geotechnical

engineer will perform the required testing and observation during grading and construction. It is the owner's responsibility to make the necessary arrangements for these required services.

4. Where referenced in this report, Percent Relative Compaction and Optimum Moisture Content shall be based on ASTM Test Designation D1557-91.

5. Areas to be graded or to receive building foundations should be cleared of obstructions including loose fill, debris, foundations, trees not designated to remain and their principal roots, or other unsuitable material. Existing depressions or voids created during site clearing should be backfilled with engineered fill.

6. Engineered fill should be placed in thin lifts not exceeding 8 inches in loose thickness, moisture conditioned, and compacted to a minimum of 90 percent relative compaction. The upper 8 inches should be compacted to a minimum of 95 percent relative compaction. Engineered fill should be keyed and benched into the hillside. A typical keying and benching detail is attached in the appendix.

7. Areas to receive engineered fill should be scarified to a depth of 6 inches, moisture conditioned, and compacted to a relative compaction of 90 percent.

8. The on-site silty sand may be reused as engineered fill once the majority of organics and other deleterious material is removed.

9. Any imported fill should meet the following criteria:

   a. Be free of wood, brush, roots, grass, debris and other deleterious materials.

   b. Not contain rocks or clods greater than 2.5 inches in diameter.

   c. Not more than 20 percent passing the #200 sieve.

   d. Have a plasticity index less than 15.

   e. Be approved by the geotechnical engineer. Submit to the geotechnical engineer samples of import material or utility trench backfill for compliance testing a minimum of 4 days before it is delivered to the job site.

10. Temporary cut-slopes may be cut at a maximum gradient of 1:1 (H:V) under dry conditions. Permanent cut should be sloped no steeper than 2 ½:1 (H:V). Engineered fill slopes may be graded at slopes no steeper than 2:1.

11. After the earthwork operations have been completed and the geotechnical engineer has finished his observation of the work, no further earthwork operations shall be performed except with the approval of and under the observation of the geotechnical engineer.

12. The native soil has a high potential for erosion. Disturbed slopes should be erosion controlled.

Conventional Shallow Foundations

The proposed structures may be founded on a minimum of 18 inches of engineered fill. The base of all footings should be located a minimum of 10 feet from daylight measured horizontally.

13. The proposed structures may be supported on conventional spread footings founded on a minimum of 18 inches of engineered fill as outlined in the grading section of this report. Footing dimensions should be determined in accordance with anticipated use and applicable design standards, but should be a minimum of 15 inches wide and be embedded a minimum of 12 inches for one-story structures and 18 inches for two-story structures. Footings should be stepped up the hillside horizontally. The footings should be reinforced as required by the structural designer based on the actual loads transmitted to the foundation.

14. Foundations designed in accordance with the above may be designed for an allowable soil bearing pressure of 2,000 psf for dead plus live loads. This value may be increased by one-third to include short-term seismic and wind loads.

15. Lateral load resistance for the buildings supported on footings may be developed in friction between the foundation bottom and the supporting subgrade. A friction coefficient of 0.35 is considered applicable. Passive resistance of 250 pcf may be used below a depth of 12 inches against engineered fill or in-situ silty sand.

Retaining Walls and Lateral Pressures

16. Retaining walls should be designed to resist the lateral earth pressures listed in Table 1. The values listed in Table 1 are for non-seismic conditions and are based on the assumption that walls will be adequately drained.

Table 1 - Active and At-Rest Pressures

Backslope Gradient	Active Pressure (pcf)	At-Rest Pressure (pcf)
Level	37 ½	55
2:1	45	65

17. Active pressures should be used for walls where horizontal movement at the top of the wall is not restricted. At-rest pressures should be used to design walls with movement restrained at the top, such as basement walls and walls structurally connected at the top. The walls should also be designed to resist one half of any surcharge loads imposed on the backfill behind the walls. The designer should account for the surcharge loading created during backfill operations.

18. To account for seismic loading, a horizontal line load surcharge equal to $10H^2$ lbs/horizontal foot of wall may be assumed to act at 0.6H above the heel of the wall base (where H is the height of the wall.)

19. The above lateral pressures assume the walls are fully drained to prevent hydrostatic pressure behind the walls. Drainage materials behind the wall should consist of Class 1, Type A permeable material complying with Section 68 of CalTrans Standard Specifications, latest edition, or 3/4 inch permeable drainrock wrapped in Mirafi 140 N or equivalent. The drainage material should be at least 12 inches thick. The drains should extend from the base of the walls to within 12 inches of the top of the backfill. A perforated pipe should be placed (holes down) about 4 inches above the bottom of the wall and discharge at a suitable location. Wall backdrains should be plugged at the surface with clayey material to prevent infiltration of surface runoff into the backdrains.

1997 UBC Seismic Design Considerations

For purposes of design of structural features for the proposed project seismic coefficients may be used based on a soil profile Sd as described in Table 16-J of the 1997 UBC. The coefficients should be based on the 1997 UBC and the Monterey Bay-Tularcitos Fault (Type B at a distance of 4 kilometers).

Slabs-on-Grade

20. Concrete slabs-on-grade planned for the site should be constructed on 18 inches of engineered fill as outlined in the grading section of this report. Prior to construction of the slab, the subgrade surface should be proof-rolled to provide a smooth, firm, uniform surface for slab support. Slab reinforcement should be provided in accordance with the anticipated use and loading of the slab. As a minimum, we recommend the use of number 4 bars placed within the slab at 18 inches on center. Slab joints should be spaced no more than 12 feet on center to minimize random cracking. While some movement of slabs is likely, a well-prepared subgrade including pre-moistening prior to pouring concrete, adequately spaced expansion joints, and good workmanship should minimize cracking and movement.

21. In areas where floor wetness would be undesirable, a blanket of 4 inches of free-draining gravel should be placed beneath the floor slab to act as a capillary break. In order to minimize vapor transmission, an impermeable membrane should be placed over the gravel. The membrane should be covered with 2 inches of sand or rounded gravel to protect it during construction. The sand or gravel should be lightly moistened just prior to placing the concrete to aid in curing the concrete. If moisture is expected a surface treatment or moisture retardant should be added to the concrete.

Site Drainage

22. Proper control of drainage will be essential to the project. The on-site soil is considered to be highly erosive.

23. Surface drainage should include positive gradients so that surface runoff is not permitted to pond adjacent to foundations, slabs or retaining walls. Surface drainage should be directed away from building foundations. The slope from the foundation elements should be 2 percent.

24. Full roof gutters and downspouts should be placed around eaves. Discharge from the roof gutters should be conveyed away from the building site via closed plastic conduit released into level T-spreaders at an appropriate location.

25. The migration of water or spread of extensive root systems below foundations, slabs, or pavements may cause undesirable differential movements and subsequent damage to these structures. Landscaping should be planned accordingly.

Plan Review, Construction Observation, and Testing

26. Our firm must be provided the opportunity for a general review of the final project plans prior to construction so that our geotechnical recommendations may be properly interpreted and implemented. If our firm is not accorded the opportunity of making the

recommended review, we can assume no responsibility for misinterpretation of our recommendations. We recommend that our office review the project plans prior to submittal to public agencies, to expedite project review. The recommendations presented in this report require our review of final plans and specifications prior to construction and upon our observation and, where necessary, testing of the earthwork and foundation excavations. Observation of grading and foundation excavations allows anticipated soil conditions to be correlated to those actually encountered in the field during construction.

Project No. M8934
9 September 2005

LIMITATIONS AND UNIFORMITY OF CONDITIONS

1. The recommendations of this report are based upon the assumption that the soil conditions do not deviate from those disclosed in the borings. If any variations or undesirable conditions are encountered during construction, or if the proposed construction will differ from that planned at the time, our firm should be notified so that supplemental recommendations can be given.

2. This report is issued with the understanding that it is the responsibility of the owner, or his representative, to ensure that the information and recommendations contained herein are called to the attention of the Architects and Engineers for the project and incorporated into the plans, and that the necessary steps are taken to ensure that the Contractors and Subcontractors carry out such recommendations in the field. The conclusions and recommendations contained herein are professional opinions derived in accordance with current standards of professional practice. No other warranty expressed or implied is made.

3. The findings of this report are valid as of the present date. However, changes in the conditions of a property can occur with the passage of time, whether they be due to natural processes or to the works of man, on this or adjacent properties. In addition, changes in applicable or appropriate standards occur whether they result from legislation or the broadening of knowledge. Accordingly, the findings of this report may be invalidated, wholly or partially, by changes outside our control. Therefore, this report should not be relied upon after a period of three years without being reviewed by a geotechnical engineer.

Project No. M8934
9 September 2005

APPENDIX A

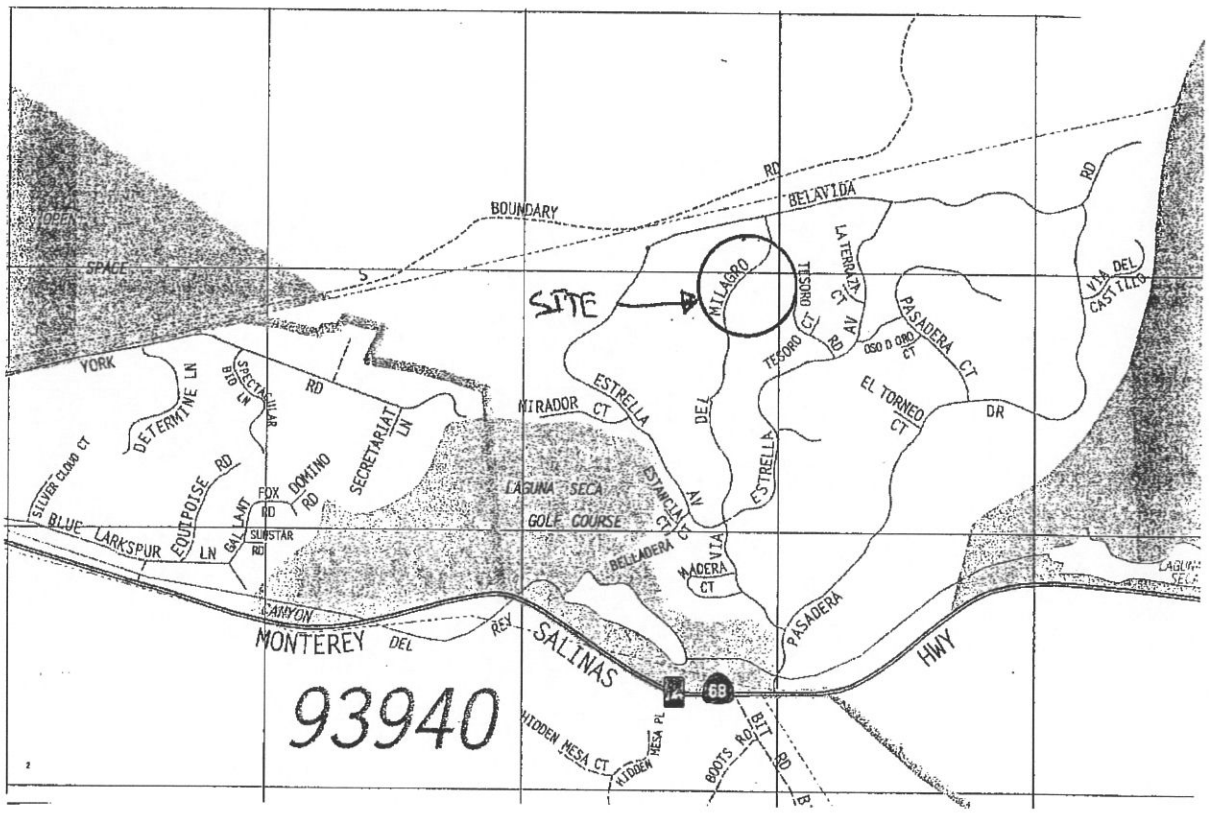
Vicinity Map

Site Plan

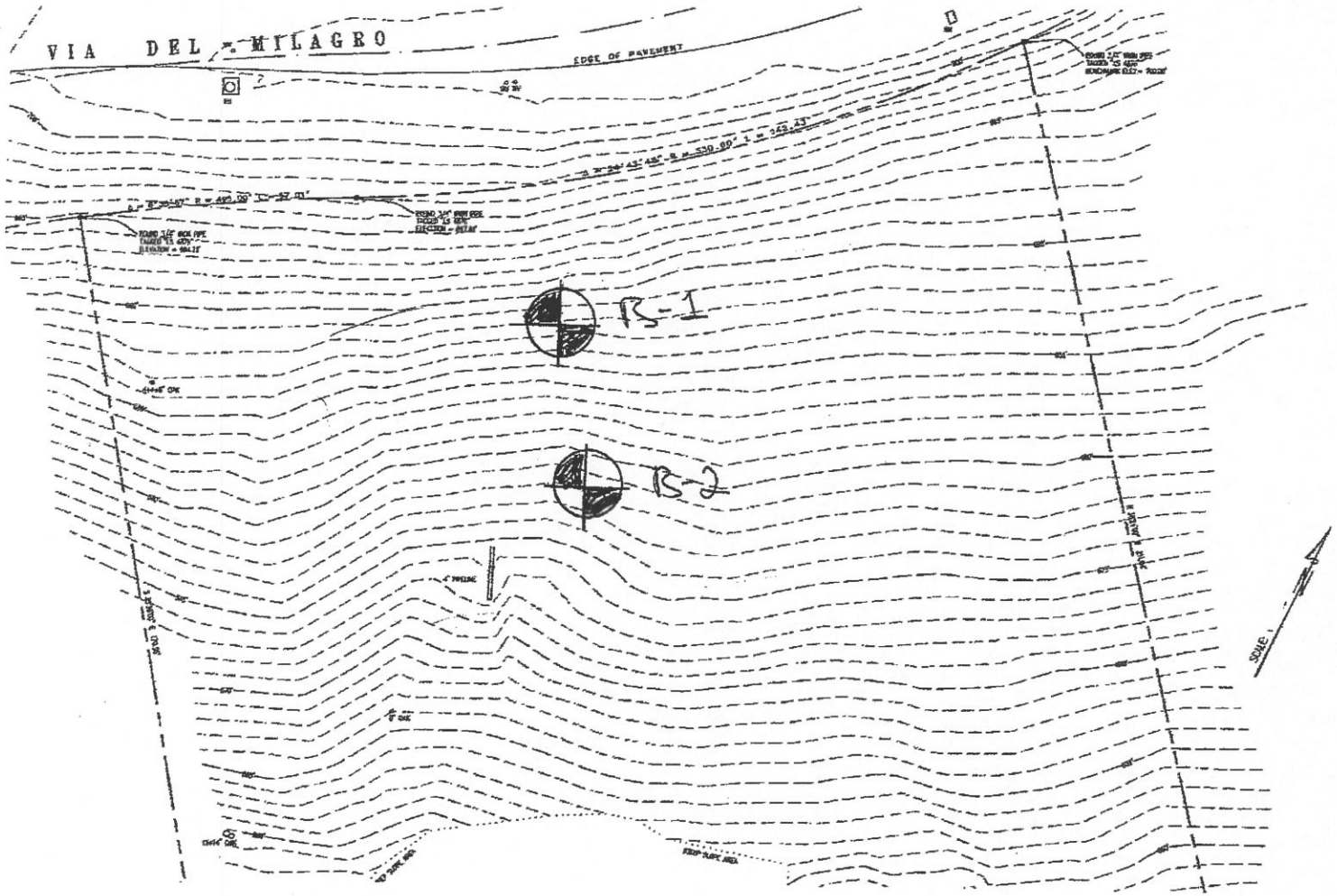
Logs of Test Borings

Laboratory Test Results

Keying and Benching Detail



PROJECT NO: M8934	VICINITY MAP
DATE: 9-7-04	401 Via Del Milagro
SCALE:	Monterey County, California
DRAWN BY:	
HARO, KASUNICH & ASSOCIATES	FIGURE NO. 1



Modified from Topographic Survey by Monterey Bay Engineers dated December 2004.

Approximate Boring Location

PROJECT NO:	M8934
DATE:	9-7-05
SCALE:	N.T.S.
DRAWN BY:	

Boring Site Plan

401 Via Del Milagro
Monterey County, California

HARO, KASUNICH & ASSOCIATES

FIGURE NO. 2

Haro, Kasunich & Associates, Inc.
Geotechnical & Environmental Engineers

Lot 98 - Pasadera

PROJECT NO. M8934

LOGGED BY GB | DATE DRILLED June 23, 2005 | BORING DIAMETER 5" | BORING NO. B-1

Depth, ft.	Sample No. and type	Symbol	SOIL DESCRIPTION	Unified Soil Classification	Blows/foot 350 ft - lbs.	Qu - t.s.f. Penetrometer	Dry Density p.c.f.	Moisture % dry wt.	MISC. LAB RESULTS
0			Yellow brown poorly graded Sand with Silt, loose, slightly damp	SP-SM					
	1-1 (T)				7			4.5	Passing #200 Sieve = 10%
5	1-2 (T)				7			5.8	
	1-3 (T)		Medium dense		19			10.2	
10			Boring terminated at 9.5 feet						
15									
20									
25									
30									
35									

SuperLog CivilTech Software, USA www.civiltech.com File: C:\Superlog4\HKALOGS\M8934.log Date: 9/9/05

HARO, KASUNICH AND ASSOCIATES, INC.

BY: DK | FIGURE NO. 3

Haro, Kasunich & Associates, Inc.
Geotechnical Consultants

Lot 98 - Pasadera

PROJECT NO. M8934

LOGGED BY GB DATE DRILLED June 23, 2005 BORING DIAMETER 5" BORING NO. B-2

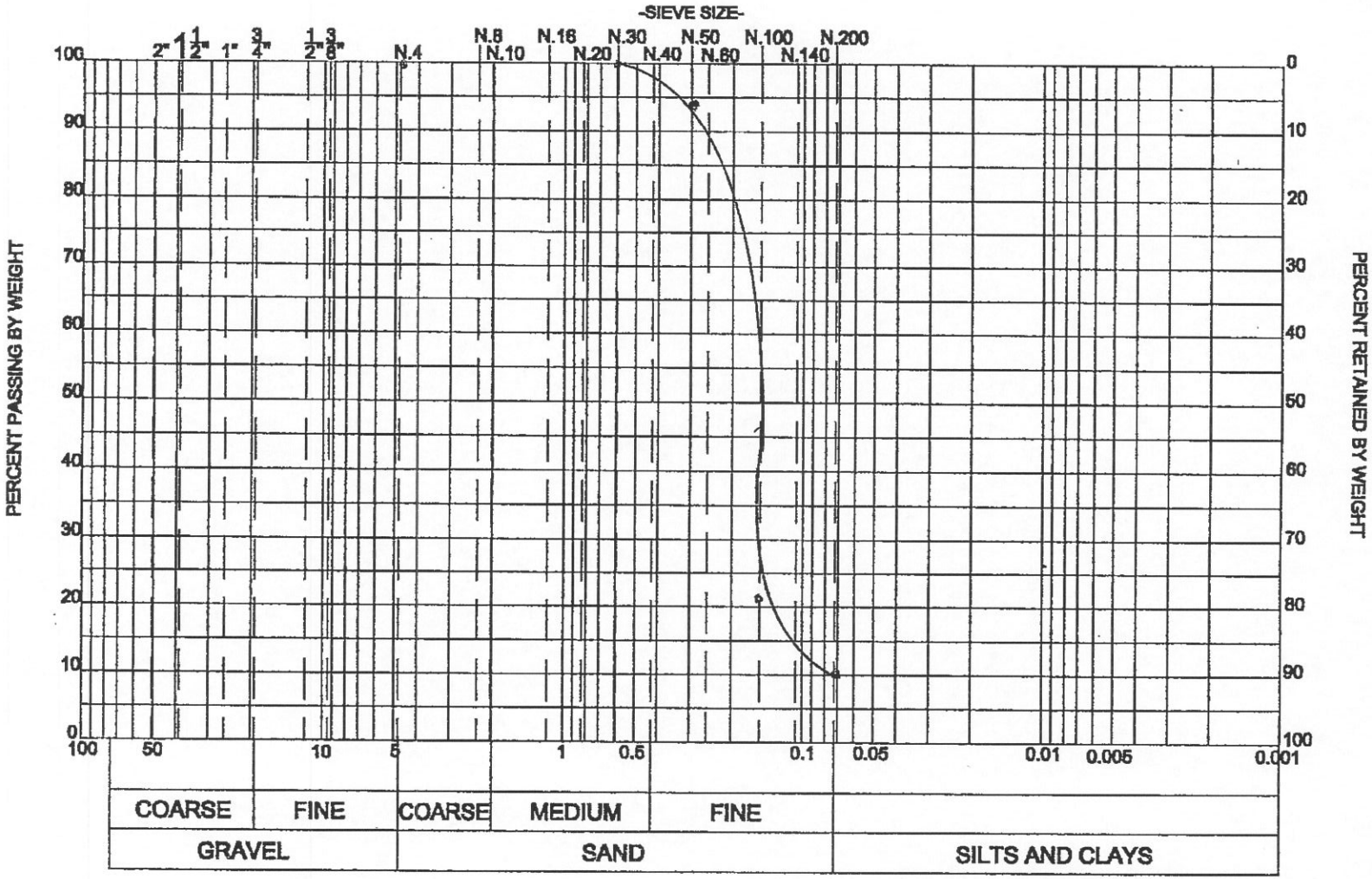
Depth, ft.	Sample No. and type	Symbol	SOIL DESCRIPTION	Unified Soil Classification	Blows/foot 350 ft - lbs.	Qu - t.s.f. Penetrometer	Dry Density p.c.f.	Moisture % dry wt.	MISC. LAB RESULTS
0			Yellow brown poorly graded Sand with Silt, loose, slightly damp	SP-SM					
	2-1 (T)				11			4.7	
5	2-2 (T)		Loose		10			6.4	
	2-3 (T)		Loose		8			9.9	
10	2-4 (T)		Medium dense		28			10.1	
15			Boring terminated at 15.5 deep						
20									
25									
30									
35									

SuperLog CivilTech Software, USA www.civiltech.com File: C:\Superlog4\HKALOGS\M8934.log Date: 9/9/05

HARO, KASUNICH AND ASSOCIATES, INC.

BY: DK

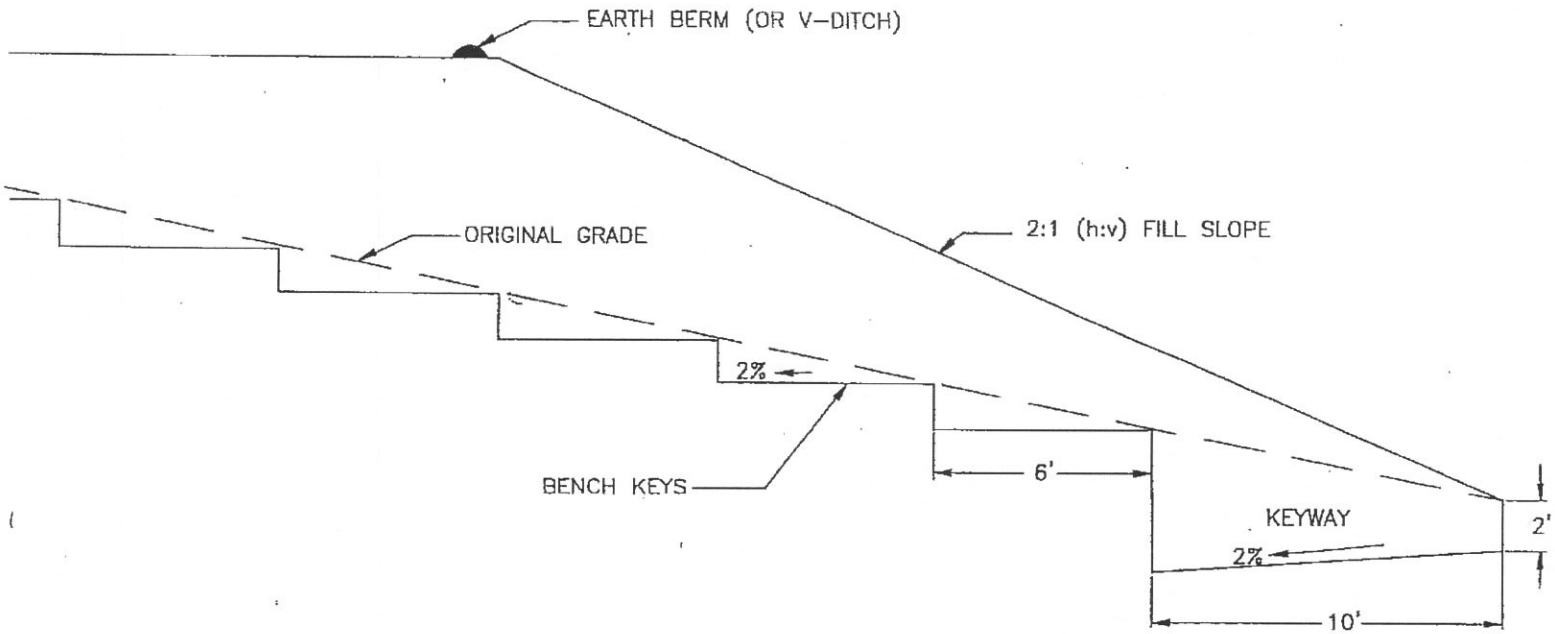
FIGURE NO. 4



Project No.:	% GRAVEL		DESCRIPTION: Poorly Graded Sand with Silt
Date:	% SAND	90	
Sample No.:	% SILT & CLAY	10	
Drawn By:			

HARO, KASUNICH & ASSOCIATES

Fig. 5



NOTES

1. FOR SLOPES GREATER THAN 20' HIGH, AN INTERMEDIATE BENCH AT LEAST 8 FEET WIDE SHALL BE CONSTRUCTED
2. REFER TO GEOTECHNICAL REPORT FOR COMPACTION REQUIREMENTS.
3. DOWNSLOPE SIDE OF KEYWAY TO BE AT LEAST 2 FEET DEEP BELOW NATIVE GRADE, TO BE VERIFIED BY GEOTECHNICAL ENGINEER
4. DRAWING IS NOT TO SCALE

TYPICAL KEYWAY DETAIL
FILL SLOPE CONSTRUCTION REQUIREMENTS

HARO, KASUNICH & ASSOCIATES INC. | FIGURE NO. 6

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